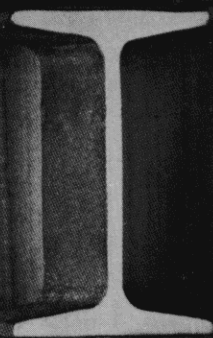
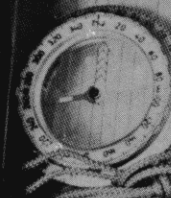
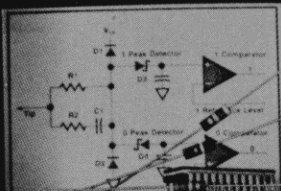


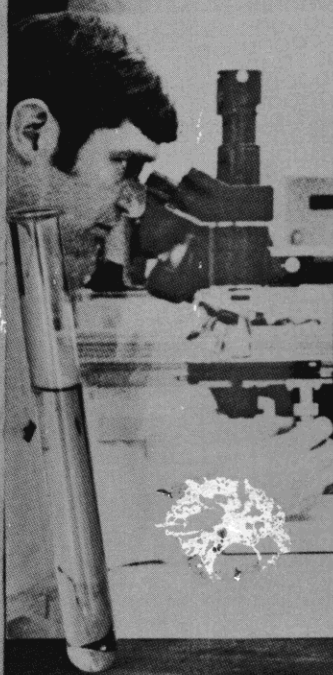
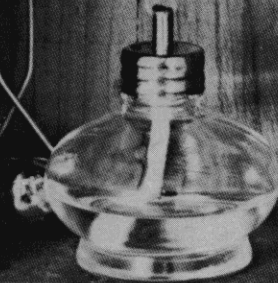
Start Zone

81	71	61	51	41	31	21
92	82	72	62	52	42	32
103	93	83	73	63	53	43
114	104	94	84	74	64	54
125	115	105	95	85	75	65
136	126	116	106	96	86	76
147	137	127	117	107	97	87
158	148	138	128	118	108	98



Hewlett-Packard HP-19C/HP-29C SOLUTIONS

CIVIL ENGINEERING



INTRODUCTION

This HP-19C/HP-29C Solutions book was written to help you get the most from your calculator. The programs were chosen to provide useful calculations for many of the common problems encountered.

They will provide you with immediate capabilities in your everyday calculations and you will find them useful as guides to programming techniques for writing your own customized software. The comments on each program listing describe the approach used to reach the solution and help you follow the programmer's logic as you become an expert on your HP calculator.

You will find general information on how to key in and run programs under "A Word about Program Usage" in the Applications book you received with your calculator.

We hope that this Solutions book will be a valuable tool in your work and would appreciate your comments about it.

The program material contained herein is supplied without representation or warranty of any kind. Hewlett-Packard Company therefore assumes no responsibility and shall have no liability, consequential or otherwise, of any kind arising from the use of this program material or any part thereof.

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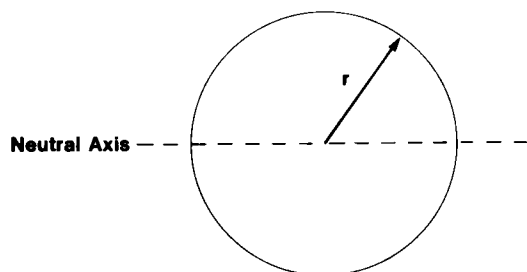
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	This program performs an interchangeable solution for four properties of circular sections: moment of inertia; diameter; polar moment of inertia; and area.	
2.	PROPERTIES OF RECTANGULAR SECTIONS	4
	This program performs an interchangeable solution for three properties of rectangular sections: moment of inertia; width; and height. The polar moment of inertia and section area are also calculated.	
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	This program provides an interchangeable solution for three properties of annular sections: moment of inertia; outside diameter; and inside diameter. The polar moment of inertia and section area are also calculated.	
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1.

PROPERTIES OF CIRCULAR SECTIONS

1.

This program performs an interchangeable solution for four properties of circular sections. Given either the moment of inertia I, diameter d, polar moment of inertia J, or area A, the remaining properties can be calculated.



EQUATIONS:

$$I = \frac{\pi d^4}{64}$$

$$J = \frac{\pi d^4}{32}$$

$$A = \frac{\pi d^2}{4}$$

EXAMPLE 1:

If the moment of inertia of a section must be 60 in., what is the necessary diameter? What is the polar moment of inertia? What is the area?

EXAMPLE 2:

The diameter of a section is 10 centimeters. What is the moment of inertia? What is the polar moment of inertia? What is the area?

SOLUTIONS:

- 1.
- | | | |
|--------|------|-----------------------|
| 60.00 | GSB1 | |
| 60.00 | *** | (in. ⁴) I |
| | R↓ | |
| 120.00 | *** | (in. ⁴) J |
| | R↓ | |
| 27.46 | *** | (in. ²) A |
| | R↓ | |
| 5.91 | *** | (in.) d |
- 2.
- | | | |
|--------|------|----------------------|
| 10.00 | GSB4 | |
| 490.87 | *** | (cm ⁴) I |
| | R↓ | |
| 981.75 | *** | (cm ⁴) J |
| | R↓ | |
| 78.54 | *** | (cm ²) A |
| | R↓ | |
| 10.00 | *** | (cm) d |

Program Listings

3

01 *LBL1			
02 Pi	I = J/2		
03 ÷			
04 JX			
05 2			
06 x			
07 GT09			
08 *LBL2	J		
09 2			
10 ÷			
11 GT01			
12 *LBL3	A		
13 Pi			
14 ÷			
15 GT09			
16 *LBL4			
17 X²			
18 4			
19 ÷			
20 *LBL9	d²/4		
21 ST01			
22 ST02			
23 ST×1			
24 4			
25 ST÷1			
26 x	d		
27 JX			
28 RCL2			
29 Pi			
30 ST×1	A		
31 x			
32 RCL1			
33 2	J		
34 x	I		
35 RCL1	*** I J A d		
36 R/S			

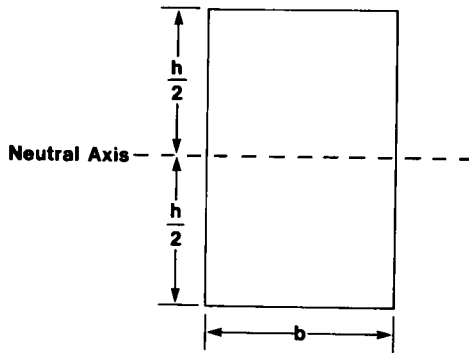
*** "Print Stack" may be inserted before "R/S".

REGISTERS					
0	1 Used	2 d²/4	3	4	5
6	7	8	9	10	11
12	13	14	15	16	17
18	19	20	21	22	23
24	25	26	27	28	29

PROPERTIES OF RECTANGULAR SECTIONS

2.

This program performs an interchangeable solution for the moment of inertia I , the width b and the height h of a rectangular section. When b and h are known, the polar moment of inertia J and the section area can also be found.



SOLUTION:

```

5.00 ENT↑
3.00 ENT↑
0.00 GSB1
31.25 *** (in.4)
      R/S
15.00 *** (in.2)
      R/S
42.50 *** (in.4)
5.00 ENT↑
0.00 ENT↑
40.00 GSB1
3.84 *** (in.)
  
```

EQUATIONS:

$$I = \frac{bh^3}{12}$$

$$J = \frac{bh(b^2 + h^2)}{12}$$

$$A = bh$$

REMARKS:

Values of polar moment of inertia J calculated by this program must not be used to calculate torsional stress and strain in rectangular members.

EXAMPLE:

What is the moment of inertia of a section with $b=3$ and $h=5$? What is the polar moment of inertia? What is the area? What would b have to be if $I=40$?

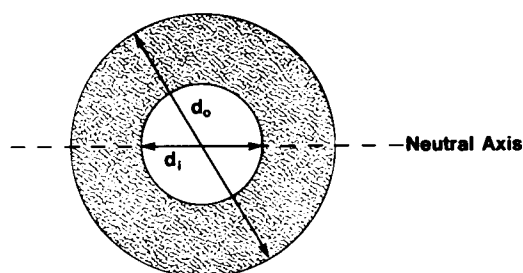
Program Listings

01 *LBL1		48 →P	
02 1		49 X2	
03 2		50 RCL2	
04 ST04		51 RCL3	
05 R↓		52 x	
06 ST01	I	53 R/S	** A
07 R↓		54 x	
08 ST02	b	55 RCL4	
09 X=Y		56 ÷	*** J
10 ST03	h	57 R/S	
11 X=0?			
12 GT09	Calculate h		
13 R↓			
14 X=0?			
15 GT08			
16 RCL2	Calculate b		
17 RCL3	Calculate I		
18 3			
19 YX			
20 x			
21 RCL4			
22 ÷			
23 GT00	I		
24 *LBL9			
25 RCL1			
26 RCL4			
27 x			
28 RCL2			
29 ÷			
30 3			
31 1/X			
32 YX			
33 ST03	h		
34 GT00			
35 *LBL8		** "Printx" may replace "R/S".	
36 RCL1		*** "Printx" may be inserted before "R/S".	
37 RCL4			
38 x			
39 RCL3			
40 3			
41 YX			
42 ÷			
43 ST02	b		
44 *LBL0	** I, h, or b		
45 R/S			
46 RCL2			
47 RCL3			

REGISTERS					
0	1 I	2 b	3 h	4 12	5
6	7	8	9	.0	.1
.2	.3	.4	.5	16	17
18	19	20	21	22	23
24	25	26	27	28	29

PROPERTIES OF ANNULAR SECTIONS

This program provides an interchangeable solution for the moment of inertia I , the outside diameter d_o , and the inside diameter d_i of an annular section. Once d_o and d_i are known, the polar moment of inertia J and the area of the section can be calculated.



EQUATIONS:

$$I = \frac{\pi(d_o^4 - d_i^4)}{64}$$

$$J = \frac{\pi(d_o^4 - d_i^4)}{32} = 2I$$

$$A = \frac{\pi(d_o^2 - d_i^2)}{4}$$

EXAMPLE:

If d_i equals 3 inches and I equals 10 in^4 , what is d_o ? What is A ?

What would I be if d_o equals 4.5 inches?

SOLUTION:

```

3.00 ENT↑
0.00 ENT↑
10.00 GSB1
4.11 *** d_o (in.)
      R↓
3.00 *** d_i (in.)
      R↓
10.00 *** I (in.4)
2.00 x
20.00 *** J (in.4)
      R↓
6.18 *** A (in.2)

```

```

3.00 ENT↑
4.50 ENT↑
0.00 GSB1
4.50 ***
      R↓
3.00 ***
      R↓
16.15 *** I (in.4)

```


Program Listings

9

01 *LBL1		48 RCL3	
02 ST03	I	49 GSB0	
03 R4	d ₀	50 ST01	
04 ST01		51 GT07	
05 XZY	d _i	52 *LBL0	
06 ST02		53 6	
07 X=0?	Calculate d _i	54 4	
08 GT09		55 x	
09 R4	Calculate d ₀	56 Pi	
10 X=0?	Calculate I	57 ÷	
11 GT08		58 RCL1	
12 RCL1		59 4	
13 4		60 Yx	
14 Yx		61 +	
15 RCL2		62 4	
16 4		63 1/X	
17 Yx		64 Yx	
18 -		65 RTN	
19 Pi			
20 x			
21 6			
22 4			
23 ÷			
24 ST03			
25 *LBL7			
26 RCL1			
27 X²			
28 RCL2			
29 X²			
30 -			
31 Pi			
32 x			
33 4			
34 ÷			
35 RCL3			
36 RCL2			
37 RCL1			
38 R/S			
39 *LBL9			
40 RCL3			
41 CHS			
42 GSB0			
43 ST02			
44 GT07			
45 *LBL8			
46 RCL2			
47 ST01			

A

*** d₀ d_i I A

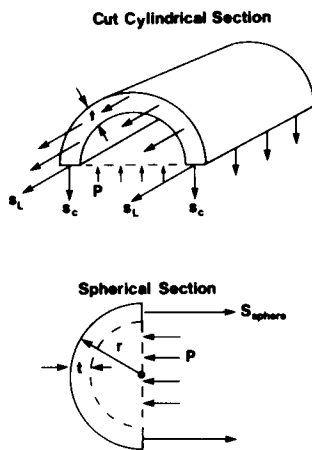
*** "Print Stack" may be inserted
before "R/S".

REGISTERS

0	1	2	3	4	5
6	7	8	9	10	11
12	13	14	15	16	17
18	19	20	21	22	23
24	25	26	27	28	29

THIN-WALLED PRESSURE VESSELS

This program can be used to correlate diameter, stress, pressure and thickness for cylindrical and spherical pressure vessels. Either the hoop stress s_c or the longitudinal stress s_L may be input for cylinders. For spheres, only the hoop stress s_{sphere} is applicable.



REMARKS:

The thickness of the walls must be negligible with respect to the value of the radius. The equations are not valid in the neighborhood of end closures for cylindrical vessels.

EXAMPLE 1:

A basketball has a diameter of 9.3 inches. The thickness of the cord layer which resists virtually all of the internal pressure is 1/32 inch. The recommended pressure is 9 pounds per square inch. What is the stress in the cord layer?

EXAMPLE 2:

A four inch diameter pipe contains steam at 1000 pounds per square inch. What thickness is required if hoop stress is not to exceed 15000 pounds per square inch?

EQUATIONS:

for hoop stress in cylinders: $s_c = \frac{Pr}{t}$

for longitudinal stress in cylinders:
 $s_L = \frac{Pr}{2t}$

for hoop stress in spheres: $s_{\text{sphere}} = \frac{Pr}{2t}$

where:

P is internal pressure;

D is diameter of vessel ($r=D/2$);

t is thickness of vessel

SOLUTIONS:

1. 9.30 ENT↑
 0.00 ENT↑
 9.00 ENT↑
 32.00 1/X
 GSB1
 669.60 *** (psi)

2. 4.00 ENT↑
 15000.00 ENT↑
 2.00 ÷ $s_c/2$
 1000.00 ENT↑
 0.00 GSB1
 0.13 *** (in)

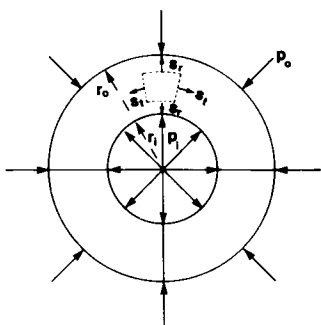
[illegible]

Program Listings

01 *LBL1					
02 ST04	t				
03 R↓	P				
04 ST03					
05 R↓					
06 ST02	$s_c/2$ or s_L or s_s				
07 R↓					
08 4					
09 ÷	D/4				
10 ST01					
11 X=0?	Calculate D				
12 GT09					
13 R↓					
14 X=0?	Calculate t				
15 GT08					
16 R↓					
17 X=0?	Calculate P				
18 GT07	Calculate $s_c/2$ or				
19 RCL1	s_L or s_s				
20 RCL3					
21 x					
22 RCL4					
23 GT00					
24 *LBL9					
25 RCL4					
26 RCL2					
27 x					
28 4					
29 x					
30 RCL3					
31 GT00					
32 *LBL8					
33 RCL1					
34 RCL3					
35 x					
36 RCL2					
37 GT00					
38 *LBL7					
39 RCL4					
40 RCL2					
41 x					
42 RCL1					
43 *LBL0					
44 ÷					
45 R/S	*** D, S_x , P, or t			*** "Printx" may be inserted before "R/S".	
REGISTERS					
0	1 D/4	2 S_x	3 P	4 t	5
6	7	8	9	.0	.1
.2	.3	.4	.5	16	17
18	19	20	21	22	23
24	25	26	27	28	29

STRESS IN THICK-WALLED CYLINDERS

This program calculates the radial and tangential components of normal stress for thick-walled, cylindrical, pressure vessels.



EQUATIONS:

$$s_r = \frac{r_i^2 P_i - r_o^2 P_o}{r_o^2 - r_i^2} - \frac{r_i^2 r_o^2 (P_i - P_o)}{r^2 (r_o^2 - r_i^2)}$$

$$s_t = \frac{r_i^2 P_i - r_o^2 P_o}{r_o^2 - r_i^2} + \frac{r_i^2 r_o^2 (P_i - P_o)}{r^2 (r_o^2 - r_i^2)}$$

where:

s_r is the radial component of stress;

s_t is the tangential component of stress;

r_i is the internal radius;

r_o is the outer radius;

r is the radius where calculated stresses occur;

P_i is the internal pressure;

P_o is the outside pressure.

EXAMPLE:

A cylinder has an inner radius of 1.00 inch and an outer radius of 2.00 inches. The inner pressure is 10,000 pounds per square inch and the outer pressure is 150 pounds per square inch. What are the values of radial and tangential stresses for radii of 1.00, 1.25, 1.75 and 2.00 inches?

SOLUTION:

```

1.00 GSB1
2.00 ENT↑
150.00 ENT↑
1.00 ENT↑
10000.00 R/S
-10000.00 *** s_r psi
X↑Y
16266.67 *** s_t psi
1.25 GSB1
R/S
-5272.00 *** s_r
X↑Y
11538.67 *** s_t
1.75 GSB1
R/S
-1155.10 *** s_r
X↑Y
7421.77 *** s_t
2.00 GSB1
R/S
-150.00 *** s_r
X↑Y
6416.67 *** s_t

```

REMARKS:

A negative stress indicates compression.

REFERENCE:

J.E. Shigley, Mechanical Engineering Design, McGraw Hill, 1963.

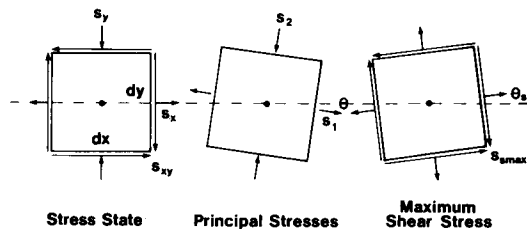
Program Listings

15

01 #LBL1					
02 ST05	r				
03 RCL4					
04 JX					
05 RCL3					
06 RCL2					
07 JX					
08 RCL1	$P_i r_i P_o r_o$				
09 R/S					
10 X \neq Y					
11 X \neq					
12 ST02					
13 R4					
14 ST01					
15 X \neq Y					
16 ST03	$P_i - P_o$				
17 -					
18 R4					
19 X \neq					
20 ST04	$r_1^2 r_o^2$				
21 x					
22 X \neq Y					
23 R4					
24 x					
25 RCL1					
26 RCL2					
27 x					
28 RCL3					
29 RCL4					
30 x					
31 -					
32 RCL4					
33 RCL2					
34 -	$r_o^2 - r_i^2$				
35 \div					
36 X \neq Y					
37 LSTX	$r_o^2 - r_i^2$				
38 RCL5					
39 X \neq					
40 x					
41 \div					
42 -	S_r				
43 ST06					
44 LSTX					
45 2					
46 x	S_t				
47 +					
48 RCL6					
49 R/S	$S_r S_t$				
REGISTERS					
0	1 P_i	2 r_1^2	3 P_o	4 r_o^2	5 r
6 S_r	7	8	9	10	11
12	13	14	15	16	17
18	19	20	21	22	23
24	25	26	27	28	29

MOHR CIRCLE FOR STRESS

Given the state of stress on an element, the principal stresses and their orientation can be found. The maximum shear stress and its orientation can also be found.



EQUATIONS:

$$s_{smax} = \sqrt{\left(\frac{s_x - s_y}{2}\right)^2 + s_{xy}^2}$$

$$s_1 = \frac{s_x + s_y}{2} + s_{smax}$$

$$s_2 = \frac{s_x + s_y}{2} - s_{smax}$$

$$\theta = \frac{1}{2} \tan^{-1} \left(\frac{2s_{xy}}{s_x - s_y} \right)$$

$$\theta_s = \frac{1}{2} \tan^{-1} - \left(\frac{s_x - s_y}{2s_{xy}} \right)$$

where:

s_{smax} is the maximum shear stress;

s_1 and s_2 are the principal normal stresses;

θ is the angle of rotation from the principal axis to the original axis;

θ_s is the angle of rotation from the axis of maximum shear stress to the original axis;

s_x is the stress in the x direction;

s_y is the stress in the y direction;

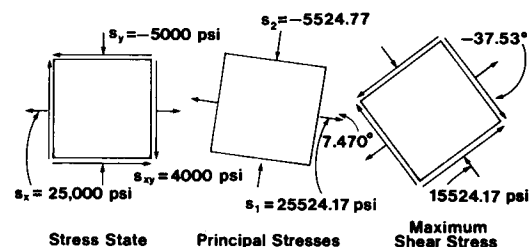
s_{xy} is the shear stress on the element.

REFERENCE:

Spotts, M.F., Design of Machine Elements, Prentice-Hall, 1971.

EXAMPLE:

If $s_x = 25000$ psi, $s_y = -5000$ psi, and $s_{xy} = 4000$ psi, compute the principal stresses and the maximum shear stress.



SOLUTION:

25000.00 ENT1

-5000.00 ENT1

4000.00 GSB1

25524.17 ***

R/S

 s_1 (psi)

-5524.17 ***

R/S

 s_2 (psi)

7.47 ***

R/S

 θ (degrees)

-37.53 ***

R/S

 θ_s (degrees)

15524.17 ***

R/S

 s_{smax} (psi)

[illegible]

Program Listings

19

01 *LBL1					
02 ENT↑					
03 R↓	$s_{xy} s_y s_x s_{xy}$				
04 ST03					
05 R↓					
06 X↔Y	s_x				
07 ST01	s_y				
08 X↔Y					
09 ST+1	$s_x - s_y$				
10 -					
11 2					
12 ST÷1					
13 ÷	$(s_x - s_y)/2$				
14 ST04					
15 →P	s_{smax}				
16 ST02					
17 RCL1					
18 +	** S1				
19 R/S	$2 \cdot \theta$				
20 X↔Y					
21 RCL1					
22 RCL2					
23 -					
24 R/S	** S2				
25 X↔Y					
26 2					
27 ÷					
28 R/S	** θ				
29 RCL4					
30 RCL3					
31 =					
32 CHS					
33 TAN↑					
34 2					
35 ÷					
36 R/S	** θ _s			** "Printx" may replace "R/S".	
37 RCL2					
38 R/S	*** s _{smax}			*** "Printx" may be inserted before "R/S".	
REGISTERS					
0	1 (s _x +s _y)/2	2 s _{smax}	3 s _{xy}	4 (s _x -s _y)/2	5
6	7	8	9	10	11
12	13	14	15	16	17
18	19	20	21	22	23
24	25	26	27	28	29

CIRCULAR PLATES WITH SIMPLY SUPPORTED EDGES

This program can be used to calculate the deflection and stress at the center of a simply supported circular plate with uniformly distributed or concentrated central loads.

EQUATIONS:

for a concentrated central load:

$$y_{\max} = \frac{(3 + \mu)Pr^2}{16\pi(1 + \mu)D}$$

$$s_{\max} = \frac{P}{h^2} \left[(1 + \mu) \left(0.485 \ln \frac{r}{h} + 0.52 \right) + 0.48 \right]$$

for a uniformly distributed load:

$$y_{\max} = \frac{(5 + \mu)Wr^4}{64D(1 + \mu)}$$

$$s_{\max} = \frac{3(3 + \mu)Wr^2}{8h^2}$$

where:

$$D = \frac{Eh^3}{12(1 - \mu^2)}$$

y_{\max} is the maximum deflection;

s_{\max} is the maximum stress;

μ is Poisson's ratio;

E is the modulus of elasticity;

h is the thickness of the plate;

r is the radius of the plate;

W is the uniformly distributed load;

P is the concentrated central load.

REFERENCES:

Spotts, M.F., Design of Machine Elements, Prentice-Hall, Inc., 1971.

REMARKS:

Deflections must be small compared to thickness of plate.

EXAMPLE 1:

Assuming that a manhole cover with an automobile tire at its center may be modeled as a simply supported flat plate with concentrated central load, what is the deflection at the center of the plate? What is the stress?

$$E = 30 \times 10^6 \text{ psi}$$

$$h = 0.75 \text{ in}$$

$$\mu = 0.3$$

$$r = 15 \text{ in}$$

$$P = 1500 \text{ lb}$$

EXAMPLE 2:

A simply supported 1/4 inch thick plate ($E = 30 \times 10^6$, $\mu = 0.3$) withstands 50 pounds per square inch. If the radius is 5 inches, what is the deflection and what is the stress at the center of the plate?

SOLUTIONS:

(1) 30.+06 ENT↑	(2) 30.+06 ENT↑
0.75 ENT↑	0.25 ENT↑
0.30 ENT↑	0.30 ENT↑
15.00 GSB1	5.00 GSB1
1500.00 GSB2	50.00 GSB3
0.01 *** (in)	0.05 *** (in)
R/S	R/S
8119.49 *** (psi)	24750.00 *** (psi)

Program Listings

01 *LBL1		48 RCL2	
02 ST01		49 1	
03 R↓		50 +	
04 ST02		51 x	
05 R↓		52 .	
06 ST03		53 4	
07 3		54 8	
08 Y*		55 +	
09 x		56 RCL5	
10 3		57 x	
11 ÷		58 RCL3	
12 1		59 X²	
13 RCL2		60 ÷	*** S _{max}
14 -		61 R/S	W
15 ÷		62 *LBL3	
16 ST04	4 D (1 + μ)	63 ST06	
17 R/S	P	64 RCL1	
18 *LBL2		65 2	
19 ST05		66 ÷	
20 RCL1		67 4	
21 X²		68 Y*	r ⁴ 16
22 x		69 x	
23 4		70 5	
24 ÷		71 GSB0	
25 P;		72 R/S	
26 ÷		73 RCL1	
27 3		74 RCL3	
28 *LBL0		75 ÷	
29 RCL2		76 X²	
30 +		77 8	
31 x		78 ÷	
32 RCL4		79 3	
33 ÷		80 x	
34 RTN	*** y _{max}	81 3	
35 RCL1		82 RCL2	
36 RCL3		83 +	
37 ÷		84 x	
38 LN		85 RCL6	
39 .		86 x	*** S _{max}
40 4		87 R/S	
41 8			
42 5			
43 x			
44 .			
45 5			
46 2			
47 +			
*** "Printx" may be inserted before "RTN" or "R/S".			

REGISTERS					
0	1 r	2 μ	3 h	4 4 D(1+μ)	5 P
6 W	7	8	9	.0	.1
.2	.3	.4	.5	16	17
18	19	20	21	22	23
24	25	26	27	28	29

CIRCULAR PLATES WITH FIXED EDGES

This program can be used to calculate the maximum deflection and stress for a circular plate with fixed edges. Either central concentrated loads or distributed loads may be input.

EQUATIONS:

$$y_{\max} = \frac{Pr^2}{16\pi D}$$

$$s_{\max} = \frac{P}{h^2} (1+\mu) \left(0.485 \ln \frac{r}{h} + 0.52 \right)$$

for distributed loads:

$$y_{\max} = \frac{Wr^4}{64D}$$

$$s_{\max} = \frac{3Wr^2}{4h^2} \quad (\text{at edge of plate})$$

where:

$$D = \frac{Eh^3}{12(1-\mu^2)}$$

y_{\max} is the maximum deflection

s_{\max} is the maximum stress;

P is the concentrated load;

W is the distributed load;

r is the radius of the plate;

h is the thickness of the plate;

μ is Poisson's ratio;

E is the modulus of elasticity.

REFERENCE:

Spotts, M.F., Design of Machine Elements, Prentice-Hall, Inc., 1971.

REMARKS:

Deflections must be small compared to the thickness of plate.

EXAMPLE 1:

The cap on a pressure vessel is a 1/4 inch thick steel plate ($E = 30 \times 10^6$ psi, $\mu = 0.3$) with a 6 inch radius. It is clamped to the opening of the pressure vessel by a ring of bolts. What are the maximum and minimum deflections and stresses in the plate if pressure cycles from 50 to 60 psi?

EXAMPLE 2:

An adjustable focal length mirror is to derive its concaved shape due to a variable force applied at its center. The mirror is chrome plated steel ($E = 30 \times 10^6$ psi, $\mu = 0.3$), 0.1 inches thick and has a radius of 12 inches. What is the deflection of the center for a force of 6.0 pounds. The edges are held securely.

SOLUTIONS:

(1)	30.+06 ENT↑	(2)	30.+06 ENT↑
	0.25 ENT↑		0.10 ENT↑
	0.30 ENT↑		0.30 ENT↑
	6.00 GSB1		12.00 GSB1
	50.00 GSB3		6.00 GSB2
	0.02 *** (in)min		FIX5
	R/S		0.00626 *** (in)
21600.00	*** (psi)		
60.00 GSB3			
0.03 *** (in)max			
	R/S		
25920.00	*** (psi)		

Program Listings

25

01 #LBL1		48 X	
02 ST01		49 RCL5	
03 R↓		50 X	
04 ST02		51 RCL3	
05 R↓		52 X²	
06 ST03		53 ÷	
07 3		54 R/S	*** s _{max}
08 YX		55 #LBL7	W
09 X		56 ST06	
10 1		57 RCL1	
11 2		58 2	
12 ÷		59 ÷	
13 1		60 4	r ⁴ /16
14 RCL2		61 YX	
15 X²		62 X	
16 -		63 4	
17 ÷		64 ÷	
18 ST04	D	65 RCL4	
19 R/S	P	66 ÷	** y _{max}
20 #LBL2		67 R/S	
21 ST05		68 RCL1	
22 RCL1		69 2	
23 4		70 ÷	
24 ÷		71 RCL3	
25 X²	r ² /16	72 ÷	r ² /4h ²
26 X		73 X²	
27 RCL4		74 3	
28 ÷		75 X	
29 P↑		76 RCL6	
30 ÷		77 X	
31 R/S	*** y _{max}	78 R/S	*** s _{max}
32 RCL1			
33 RCL3			
34 ÷			
35 LN			
36 .			
37 4			
38 8			
39 5			
40 X			
41 .			
42 5			
43 2			
44 +			
45 1			
46 RCL2			
47 +			

** "Printx" may be replace "R/S".
 *** "Printx" may be inserted before "R/S".

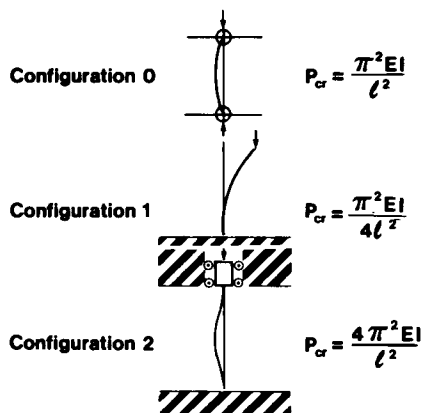
REGISTERS					
0	1 r	2 μ	3 h	4 D	5 p
6 W	7	8	9	0	.1
.2	.3	.4	.5	16	17
18	19	20	21	22	23
24	25	26	27	28	29

COMPRESSIVE BUCKLING

This program performs an interchangeable solution for the four properties of slender compression members or columns: P_{cr} , the critical buckling load; E , the modulus of elasticity; I , the minimum moment of inertia; and ℓ , the length of the member.

EQUATIONS:

Three configurations are possible, identified by the number of fixed ends on the member: 0, both ends hinged; 1, one end free and one fixed; 2, both ends fixed.



REMARKS:

Uncertainties such as the amount of restraint at the ends, eccentricity of the load, initial warp, nonhomogeneity of the material and deflection caused by lateral loads, can cause very significant changes in the behavior of a compressive member.

EXAMPLE 1:

If an 8 inch steel ($E = 30 \times 10^6$ psi) piston rod (a piston rod has zero fixed ends) must withstand a load of 15000 pounds without buckling, what moment of inertia must it have?

EXAMPLE 2:

Steel columns 40 feet long are used to support a bridge. What is the maximum load that the column can withstand without buckling? Assume 1 fixed end. $E = 30 \times 10^6$ psi, $I = 700$ in⁴.

SOLUTIONS:

(1)

```
0.00 GSB1
15000.00 ENT↑
30.+06 ENT↑
0.00 ENT↑
8.00 GSB2
3.24-03 *** I
```

(2)

```
1.00 GSB1
0.00 ENT↑
30.+06 ENT↑
700.00 ENT↑
480.00 GSB2
224893.33 *** p
```


Program Listings

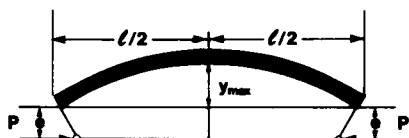
01 *LBL1	0,1, or 2	48 *LBL0	
02 5		49 RCLi	
03 ST00	i = 5	50 ÷	
04 R↓		51 R/S	*** ℓ^2 , E, I, or P
05 Pi			
06 X ²			
07 ST01			
08 R↓			
09 X=0?			
10 R/S	c = 1		
11 2			
12 X≠Y?			
13 1/X			
14 X ²	c = 1/4 or 4		
15 ST×1			
16 R/S			
17 *LBL2	ℓ I E P		
18 X ²			
19 ST05			
20 R↓			
21 ST02			
22 R↓			
23 ST03			
24 R↓			
25 ST04			
26 X=0?			
27 GT09	Calculate P		
28 DSZ	i = 4		
29 R↓			
30 X=0?			
31 GT09	Calculate ℓ^2		
32 R↓	i = 3		
33 DSZ			
34 X≠0?			
35 DSZ	Calculate E, i=2		
36 RCL4	Calculate I, i=3		
37 RCL5			
38 x			
39 RCL1			
40 ÷			
41 GT00			
42 *LBL9			
43 RCL1			
44 RCL2			
45 RCL3			
46 x			
47 x			

*** R/S may be inserted before "R/S".

REGISTERS					
0 i	1 $C\pi^2$	2 I	3 E	4 P	5 ℓ^2
6	7	8	9	.0	.1
.2	.3	.4	.5	16	17
18	19	20	21	22	23
24	25	26	27	28	29

ECCENTRICALLY LOADED COLUMNS

This program calculates the maximum deflection, the maximum moment, and the maximum stress in an eccentrically loaded column under compressive stress.



EQUATIONS:

$$y_{\max} = e \left[\sec \frac{\ell}{2} \sqrt{\frac{P}{EI}} - 1 \right]$$

$$M_{\max} = P [e + y_{\max}]$$

$$s_{\max} = \frac{P}{A} \left[1 + \frac{ecA}{I} \sec \frac{\ell}{2} \sqrt{\frac{P}{EI}} \right]$$

where:

y_{\max} is the maximum deflection;

e is the eccentricity;

ℓ is the column length;

P is the compressive load;

E is the modulus of elasticity;

I is the moment of inertia;

M_{\max} is the maximum internal moment;

s_{\max} is the maximum normal stress in the column;

c is the distance from the neutral axis of the column to the outer surface;

A is the area of the cross section

REMARKS:

Columns must be of constant cross section. Stresses may not exceed the elastic limit of the material.

REFERENCE:

Spotts, M.F., Design of Machine Elements, Prentice-Hall, 1971.

EXAMPLE:

A column 50 feet long is to support 8000 pounds. The load is to be offset 6 inches. What are the maximum values of deflection, moment, and stress in the member?

$$E = 30 \times 10^6$$

$$I = 107 \text{ in}^4$$

$$A = 7 \text{ in}^2$$

$$c = 2 \text{ in}$$

SOLUTION:

```

RAD
107.00 ST01
30.+06 ST02
50.00 ENT↑
12.00 x
ST03
6.00 ST04
8000.00 ST05
GSB1
0.74 *** (in)
GSB2
53936.76 *** (in-lb)
2.00 ENT↑
7.00 GSB3
2151.02 *** (psi)

```


Program Listings

31

01 *LBL1		48 1/X	
02 GSB0		49 x	
03 R/S	*** y_{\max}	50 1	
04 *LBL2		51 +	
05 GSB0		52 RCL5	
06 RCL4		53 RCL7	
07 +		54 ÷	
08 RCL5		55 x	
09 x		56 R/S	*** s_{\max}
10 R/S	*** M_{\max}		
11 *LBL0			
12 RCL5			
13 RCL2			
14 ÷			
15 RCL1			
16 ÷			
17 JX			
18 RCL3			
19 x			
20 2			
21 ÷			
22 COS	sec (x)		
23 1/X			
24 1			
25 -			
26 RCL4			
27 x			
28 RTN			
29 *LBL3			
30 ST07	cA		
31 x			
32 RCL4			
33 x			
34 RCL1			
35 ÷			
36 ENT↑			
37 RCL5			
38 RCL2			
39 ÷			
40 4			
41 ÷			
42 RCL1		*** "Printx" may be inserted before "R/S".	
43 ÷			
44 JX			
45 RCL3			
46 x			
47 COS			

REGISTERS

0	1 I	2 E	3 ℓ	4 e	5 p
6	7 A	8	9	.0	.1
.2	.3	.4	.5	16	17
18	19	20	21	22	23
24	25	26	27	28	29

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